

*In Situ Testing*

CPT  
&  
SPT

CPT Testing



# CPT Testing

## Three Measurements

$q_c$  = Cone-tip resistance

$u_p$  = Penetration pore pressure (piezocone)

$f_s$  = sleeve friction

## Ratios

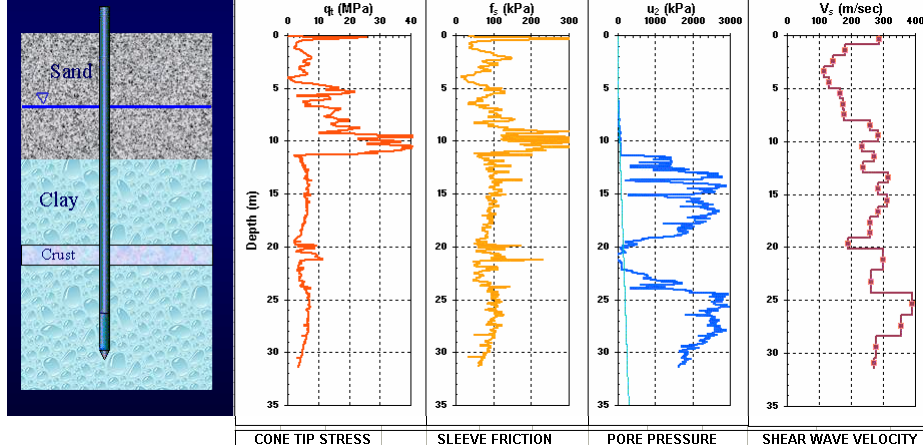
FR = friction ratio =  $f_s / q_c$

$B_q$  = pore pressure ratio =  $\frac{u_p - u_h}{q_T - \sigma'_v}$

## SCPTu Sounding, Memphis, Tennessee

Real-Time readings in computer screen

Penetration at 2 cm/s



## $q_T$ Corrected cone-tip resistance

$$q_T = q_c + (1-a_n)u_p$$

only for type 2 (type B) cones

$$a_n = 0.8 = \text{fn}(\text{cone}) \quad \text{net area ratio}$$

## CPT-parameters

$\phi'$  (Sands)

See page 129    fig. 4.16

$$\phi' = 17.6 + 11 \log \left( \frac{q_c}{(\sigma'_{vo} \cdot p_a)^{0.5}} \right) \quad \text{Kulhawy \& Mayne (1990)}$$

$$\phi' = \arctan \left[ (0.1 + 0.38 \log \left( \frac{q_c}{\sigma'_{vo}} \right)) \right]$$

$$q_{C1} = \left( \frac{q_c}{(\sigma'_{vo} \cdot p_a)^{0.5}} \right)$$

## CPT-parameters

$D_r$  = **relative density (sands)**

$$D_r = 100 \left( \frac{q_{C1}}{305 \cdot OCR^{0.2}} \right)^{0.5}$$



if unknown, use OCR = 1

$e$  = **void ratio**

$$e = 1.152 - 0.233 \cdot \log(q_{C1}) + 0.043 \log(OCR)$$

## CPT - parameters

OCR (**iterative**)

$$K_o = 0.192 \left( \frac{q_c}{p_a} \right)^{0.22} \left( \frac{\sigma'_{vo}}{p_a} \right)^{-0.31} OCR^{0.27} \quad (1)$$

$$K_o = [1 - \sin(\phi')] OCR^{\sin \phi'} \quad (2)$$

$p_a = 100 \text{ kPa}$

(1) *Mayne, CPT'95*

(2) *Mayne & Kulhawy, 1982*

a) Find  $\phi'$

b) Vary OCR until both  $K_o$  values (for eq 1 & 2) are similar

## SPT Testing



## SPT Testing

### Procedure

- 1) Drill boring (hole)
- 2) Insert SPT sampler (hollow)
- 3) Drive sampler into the ground 18" & count N of blows

to drive each 6" interval

if N for 6" > 50  
if N for all 18" > 100 } Refusal

## SPT Testing

- 4)  $N_{SPT} = \Sigma N$  for last two 6 “ intervals
- 5) Retrieve sampler & save soil
- 6) Drill to next depth & go to step 2

$N_{SPT} = \text{fn}$  (Method of drilling, how clean boring is, hammer location, hammer type, “hangover level” of technician.....)

*see p.117-118*

## SPT Testing

Efficiency corrections:

From “  $N_{SPT}$  ” to “  $N_{60}$  ”

$$N_{60} = \frac{E_m C_b C_s C_R N_{SPT}}{.60}$$

$E_m$  = hammer efficiency = fn (hammer type ) → Table 4.3

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$C_B$  = borehole dia. correction - table 4.4

$C_s$  = sampler correction - table 4.4

$C_R$  = rod length correction - table 4.4

# SPT Testing

More corrections:

$N_{SPT} \uparrow$  as  $\sigma' \uparrow$ , so

$$(N_1)_{60} = N_{60} \sqrt{\frac{p_a}{\sigma'_z}}$$

$$p_a = 100 \text{ kPa}$$

$$(N_1)_{60} \leq 2N_{60}$$

N's have been correlated with everything !!

Is One Number Enough???

$c_u$  = undrained strength

$\gamma_T$  = unit weight

$I_R$  = rigidity index

$\delta'$  = friction angle

OCR = overconsolidation

$K_0$  = lateral stress state

$e_o$  = void ratio

$V_s$  = shear wave

$E'$  = Young's modulus

$C_c$  = compression index

$q_b$  = pile end bearing

$f_s$  = pile skin friction

$k$  = permeability

$q_a$  = bearing stress

$D_R$  = relative density

$\gamma_T$  = unit weight

LI = liquefaction index

$\delta'$  = friction angle

$c'$  = cohesion intercept

$e_o$  = void ratio

$q_a$  = bearing capacity

$\sigma_p'$  = preconsolidation

$V_s$  = shear wave

$E'$  = Young's modulus

$\Psi$  = dilatancy angle

$q_b$  = pile end bearing

$f_s$  = pile skin friction